



Approval body for construction products and types of construction

Bautechnisches Prüfamt

An institution established by the Federal and Laender Governments



European Technical Assessment

ETA-05/0164 of 24 January 2017

English translation prepared by DIBt - Original version in German language

General Part

Technical Assessment Body issuing the European Technical Assessment:

Trade name of the construction product

Product family to which the construction product belongs

Manufacturer

Manufacturing plant

This European Technical Assessment contains

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of

This version replaces

Deutsches Institut für Bautechnik

fischer Highbond-Anchor FHB II

Torque controlled bonded anchor for use in concrete

fischerwerke GmbH & Co. KG Klaus-Fischer-Straße 1 72178 Waldachtal DEUTSCHLAND

fischerwerke

22 pages including 3 annexes which form an integral part of this assessment

Guideline for European technical approval of "Metal anchors for use in concrete", ETAG 001 Part 5: "Bonded anchors", April 2013, used as European Assessment Document (EAD) according to Article 66 Paragraph 3 of Regulation (EU) No 305/2011.

ETA-05/0164 issued on 22 November 2016

Deutsches Institut für Bautechnik Kolonnenstraße 30 B | 10829 Berlin | GERMANY | Phone: +49 30 78730-0 | Fax: +49 30 78730-320 | Email: dibt@dibt.de | www.dibt.de



European Technical Assessment ETA-05/0164

Page 2 of 22 | 24 January 2017

English translation prepared by DIBt

The European Technical Assessment is issued by the Technical Assessment Body in its official language. Translations of this European Technical Assessment in other languages shall fully correspond to the original issued document and shall be identified as such.

Communication of this European Technical Assessment, including transmission by electronic means, shall be in full. However, partial reproduction may only be made with the written consent of the issuing Technical Assessment Body. Any partial reproduction shall be identified as such.

This European Technical Assessment may be withdrawn by the issuing Technical Assessment Body, in particular pursuant to information by the Commission in accordance with Article 25(3) of Regulation (EU) No 305/2011.



Page 3 of 22 | 24 January 2017

Specific Part

1 Technical description of the product

The fischer Highbond-Anchor FHB II is a torque controlled bonded anchor consisting of a mortar cartridge with mortar fischer FIS HB or fischer mortar capsule FHB II–P(F) and an anchor rod FHB II - A L or FHB II - A S with hexagon nut and washer.

The glass capsule is set into a drilled hole in the concrete. The special formed anchor rod is driven into the glass capsule by machine with simultaneous hammering and turning. For the injection system the anchor rod is placed into a drilled hole filled with injection mortar. The load transfer is realised by mechanical interlock of several cones in the bonding mortar and then via a combination of bonding and friction forces in the anchorage ground (concrete).

The product description is given in Annex A.

2 Specification of the intended use in accordance with the applicable European Assessment Document

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the anchor of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic values under tension and shear load	See Annex C 1 to C 4
Displacements under tension and shear loads	See Annex C 5 and C 6

3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance
Reaction to fire	Anchorages satisfy requirements for Class A1
Resistance to fire	No performance assessed

3.3 Hygiene, health and the environment (BWR 3)

Regarding dangerous substances there may be requirements (e.g. transposed European legislation and national laws, regulations and administrative provisions) applicable to the products falling within the scope of this European Technical Assessment. In order to meet the provisions of Regulation (EU) No 305/2011, these requirements need also to be complied with, when and where they apply.

3.4 Safety in use (BWR 4)

The essential characteristics regarding Safety in use are included under the Basic Works Requirement Mechanical resistance and stability.



European Technical Assessment ETA-05/0164

Page 4 of 22 | 24 January 2017

English translation prepared by DIBt

4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

In accordance with guideline for European technical approval ETAG 001, April 2013, used as European Assessment Document (EAD) according to Article 66 Paragraph 3 of Regulation (EU) No 305/2011, the applicable European legal act is: [96/582/EC]. The system to be applied is: 1

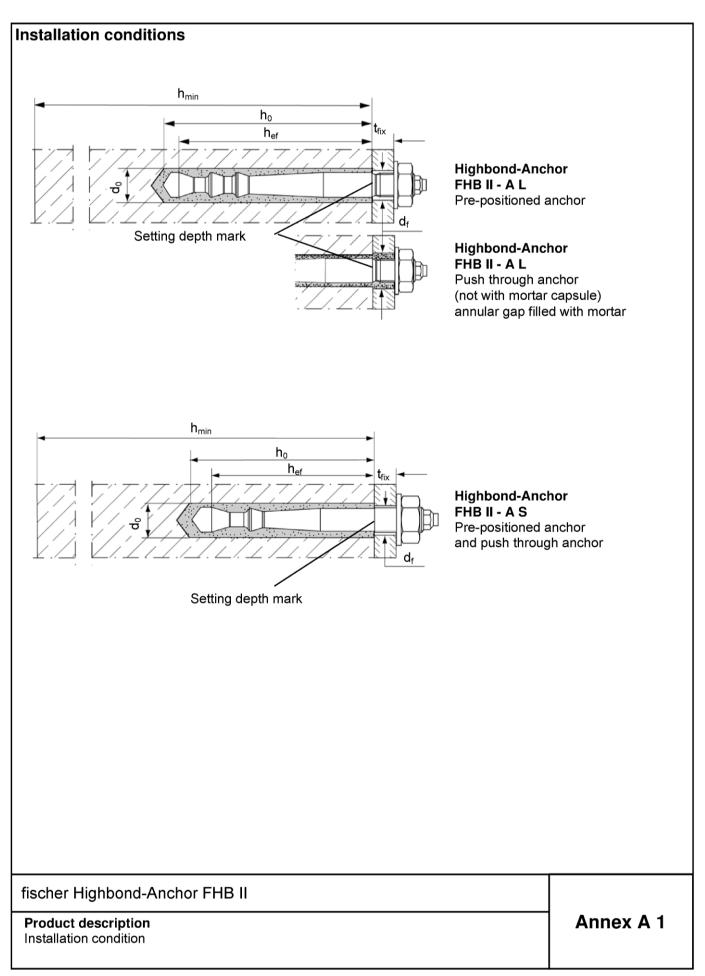
5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Document

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited at Deutsches Institut für Bautechnik.

Issued in Berlin on 24 January 2017 by Deutsches Institut für Bautechnik

Andreas Kummerow p.p. Head of Department *beglaubigt:* Baderschneider





Page 6 of European Technical Assessment ETA-05/0164 of 24 January 2017



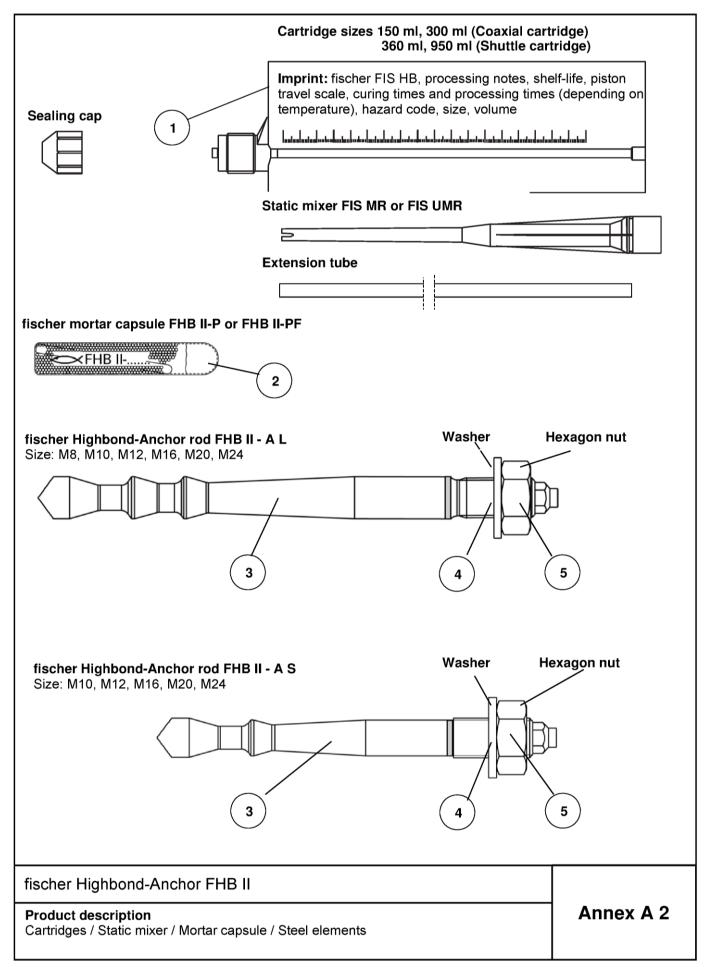




Table A1: Materials									
Part	Designation	Material							
1	Mortar cartridge	Mortar, hardener, filler							
2	Mortar capsule		Mortar, hardener, filler						
	Steel grade	Steel, zinc plated	Stainless steel A4	High corrosion resistant steel C					
3	fischer Highbond- Anchor rod FHB II - A L or FHB II - A S	Property class 8.8; EN ISO 898-1:2013 zinc plated \geq 5 µm, EN ISO 4042:1999 A2K $f_{uk} \leq$ 1000 N/mm ² $A_5 >$ 12 % fracture elongation	Property class 80 EN ISO 3506-1:2009 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362; 1.4062, 1.4662, 1.4462 EN 10088-1:2014 f _{uk} ≤ 1000 N/mm ² A ₅ > 12 % fracture elongation	Property class 80 EN ISO 3506-1:2009 1.4565; 1.4529 EN 10088-1:2014 f _{uk} ≤ 1000 N/mm ² A ₅ > 12 % fracture elongation					
4	Washer ISO 7089:2000	zinc plated ≥ 5 µm, EN ISO 4042:1999 A2K	1.4401; 1.4404; 1.4578;1.4571; 1.4439; 1.4362 EN 10088-1:2014	1.4565;1.4529 EN 10088-1:2014					
5	Hexagon nut	Property class 8; EN ISO 898-2:2012 zinc plated ≥ 5 μm, ISO 4042:1999 A2K	Property class 70 EN ISO 3506-1:2009 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362 EN 10088-1:2014	Property class 70 EN ISO 3506-1:2009 1.4565; 1.4529 EN 10088-1:2014					

fischer Highbond-Anchor FHB II

Product description Materials

Annex A 3



-		d use (part 1) nd performance categories						
Anchorages sub	oject to	fischer Injection mortar FIS HB or fischer mortar capsule FHB II-P or FHB II-PF with …						
		FHB II – A L FHB II – A S						
Hammer drilling with standard drill bit		All s	izes					
Static or quasi	uncracked concrete	all sizes	all sizes					
static load, in	cracked concrete	Tables: C1, C3, C5	Tables: C2, C4, C6					
Lieo esterem	dry or wet concrete	all s	izes					
Use category	flooded hole	all sizes (only with mortar capsule allowed)						
Kind of	-Pre positioned anchor	all s	izes					
intallation	Push through anchor	all sizes (only with injection mortar FIS HB allowed)	all sizes					
Installation temp	perature	-5 C to	+40 C					
In-service temp	erature	-40°C to +80°C (max. long term tem max. short term tem						

fischer Highbond-Anchor FHB II

Intended Use Specifications (part 1) Annex B 1



Specifications of intended use (part 2)

Base materials:

 Reinforced or unreinforced normal weight concrete Strength classes C20/25 to C50/60 according to EN 206-1:2000

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions
 (zinc coated steel, stainless steel or high corrosion resistant steel)
- Structures subject to external atmospheric exposure (including industrial and marine environment) and to permanently damp internal condition, if no particular aggressive conditions exist (stainless steel or high corrosion resistant steel)
- Structures subject to external atmospheric exposure, to permanently damp internal conditions or in other particular aggressive conditions (high corrosion resistant steel)

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used)

Design:

- · Anchorages have to be designed by a responsible engineer with experience of concrete anchor design
- Verifiable calculation notes and drawings are to be prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings (e.g. position of the anchor relative to reinforcement or to supports, etc.)
- · Anchorages under static or quasi-static actions are designed in accordance with
- EOTA ETAG 001 Annex C, 08/2010 or CEN/TS 1992-4:2009

Installation:

- Anchor installation is to be carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site
- · In case of aborted hole: The hole shall be filled with mortar
- Observe the effective anchorage depth
- Overhead installation is allowed

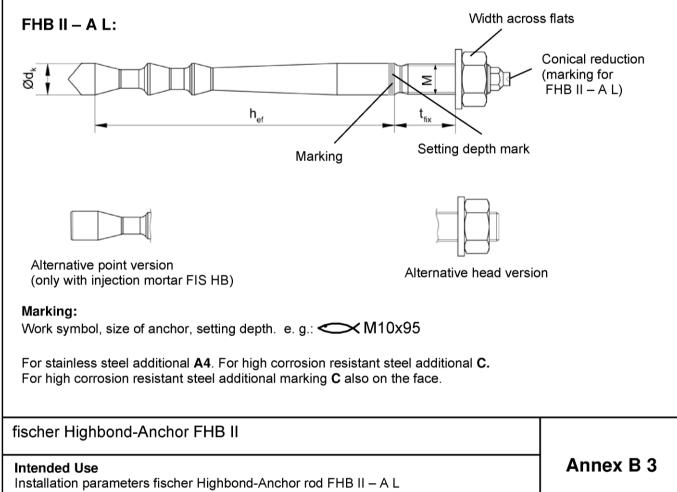
fischer Highbond-Anchor FHB II

Intended Use Specifications (part 2) Annex B 2



Table B2: Installation	parameters	for fis	cher H	ighbor	nd-And	chor ro	ods F⊢	IB II –	AL		
Size FHB II – A L			M8x 60	M10x 95	M1 100	2x 120	125	M16x 145	160	M20x 210	M24x 210
Correspondending mortar FHB II-P or FHB II-PF	capsules	[-]	8x60	95 10x95	12x 100	120 12x 120	16x 125	145 16x 145	16x 160	20x 210	24x 210
Cone diameter	d _k		9,4	10,7	12	2,5		16,8		23	,0
Width across flats	SW	1	13	17	1	9		24		30	36
Nominal drill bit diameter	d _o	1	10	12	1	4	18			25	
Drill hole depth	ho	1	75	110	115	135	140	160	175	23	35
Effective anchorage depth	h _{ef}		60	95	100	120	125	145	160	2	10
Minimum spacing and minimum edge distance	$s_{min} = c_{min}$	[mm]	4	0	5	0	55	60	70	9	0
Diameter of ancl	itioned norage d _f ≤		9	12	1	4		18		2	2
	nrough orage ²⁾ d _f ≤		11	14	1	6		20		26	
Minimum thickness of concrete member	h _{min}		100	14	10	17	70	190	220	28	30
Installation torque	T _{inst}	[Nm]	15	20	4	0		60		10	00
Thickness of fixure	t _{Fix} ≤	[mm]					1500				
1)											

¹⁾ For larger clearance holes in the fixture see EOTA ETAG 001 Annex C, 08/2010 or CEN/TS 1992-4-:2009 ²⁾ Only with mortar capsule system

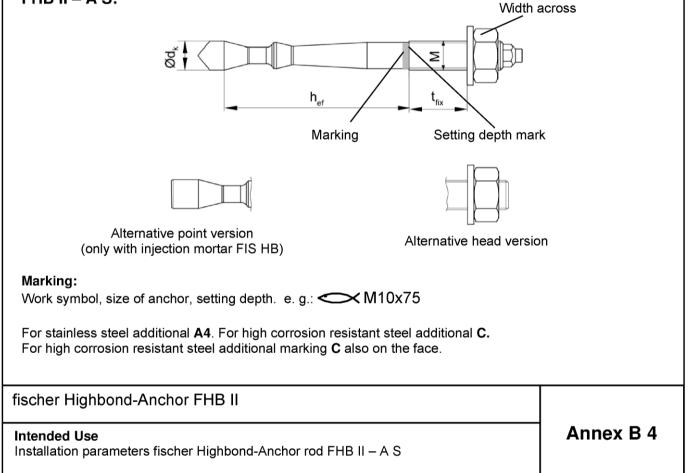




	•			M1	0x	M12x	M16x	M20x	M24x
Size FHB II – A S				60	75	75	95	170	170
Correspondending mortar capsules FHB II-P or. FHB II-PF		[-]	10x60	10x75	12x75	16x95	20x170	24x170	
Cone diameter	Cone diameter d _k			9,4		11,3	16,8	23	3,0
Width across flat	/idth across flats SW] [1	7	19	24	30	36
Nominal drill bit	minal drill bit diameter d _o] [10		12	16	25	
Drill hole depth	Drill hole depth h_0] [75	90	90	110	19	90
Effective anchorage depth	n -			60 75 75 95		17	170		
Minimum spacin minimum edge d		= c _{min}	[mm]	40			50	8	0
Diameter of clearance hole	pre-positioned anchorage	d _f ≤		12 14		14	18	22	26
in the fixture ¹⁾	push through anchorage	d _f ≤		1	2	14	18	26	
Minimum thickne of concrete mem		h _{min}		100	12	20	150	240	
Installation torqu	e	T _{inst}	[Nm]	1	5	30	50	1(00
Thickness of fixu	ire	t _{Fix} ≤	[mm]			15	00		

¹⁾ For larger clearance holes in the fixture see EOTA ETAG 001 Annex C, 08/2010 or CEN/TS 1992-4-:2009

FHB II – A S:





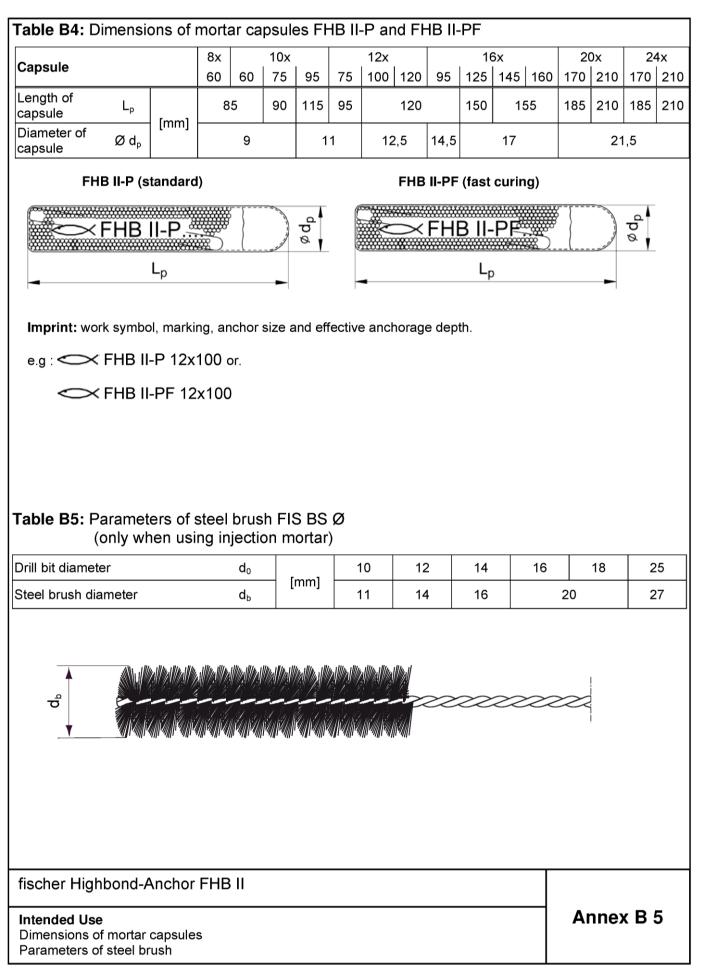




Table B6: Maximum processing time of the mortar FIS HB and minimum curing time (During the curing time of the mortar the concrete temperature may not fall below the listed minimum temperature)

System temperature	Maximum processing time	Minimum curing time ¹⁾
[°C]	t _{work} [minutes]	t _{cure} [minutes]
-5 to ±0		6 hours
> +1 to +5		3 hours
> +6 to +10	15	90
> +11 to +20	6	35
> +21 to +30	4	20
> +31 to +40	2	12

¹⁾ In wet concrete the curing times must be doubled

Table B7: Minimum curing time for mortar capsules FHB II-P and FHB II-PF (During the curing time of the mortar the concrete temperature may not fall below the listed minimum temperature)

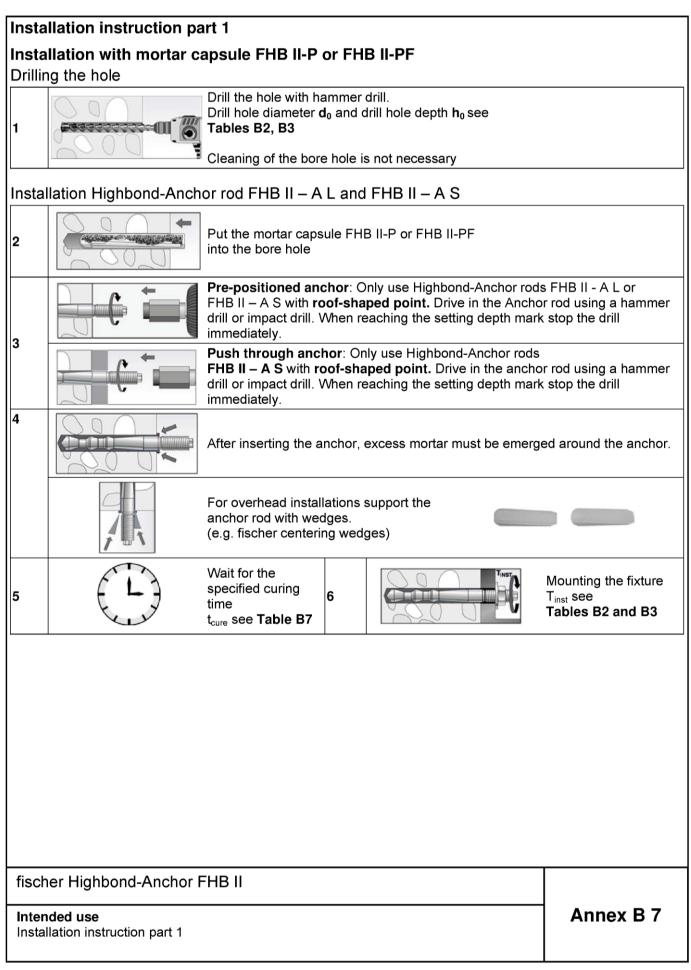
Mortar capsule	FHB II-P (standard)	Mortar capsule F	FHB II-PF (fast curing)
System temperature [°C]	Minimum curing time ¹⁾ t _{cure} [minutes]	System temperature [°C]	Minimum curing time ¹⁾ t _{cure} [minutes]
-5 to ±0	4 Stunden	-5 to ±0	8
> +1 to +10	45	> +1 to +10	6
> +11 to +20	20	> +11 to +20	4
> +20	10	> +20	2

¹⁾ In wet concrete or flooded holes the curing times must be doubled

fischer Highbond-Anchor FHB II

Intended Use Processing times and curing times Annex B 6







	allation with injection ng and cleaning the ho				
1		Drill the hole with hamme Drill hole diameter d ₀ and Tables B2, B3		n h _o see	
2	min. 2x	Blow out the drill hole twid If necessary, remove star		of the bore hole	
2	Hocher at 1	For drill hole diameter d ₀ < 25 mm with hand- blowout or oil-free compressed air	₽	$d_0 = 25$ compre	hole diameter mm with oil-free ssed air (p ≥ 6 bar) leaning nozzle.
3	min. 2x	Brush the bore hole twice Corresponding brushes see Table B5		-	
4	min. 2x	Blow out the drill hole twice.			
4		For drill hole diameter d ₀ < 25 mm with hand- blowout or oil-free compressed air	₽	$d_0 = 25$ compre	l hole diameter mm with oil-free essed air (p ≥ 6 bar) cleaning nozzle.
Prep	aring the cartridge				
5		Remove the sealing cap Screw on the static mixer (the spiral in the static mixe	er must be clea	rly visible)	
6	ficher zz		Place the cart	tridge into the dis	penser
7	X	×		y grey in colour. I	of material until the Do not use mortar that
		Observe the processing time, t _{work} see Table B6		ecessary remove	eded, use a new static e encrusted material in
fisch	ner Highbond-Anchor I	FHB II			
Inte	nded use				Annex B 8



.)-	ction of the mortar	Fill approximately 2/3 of the drill hole with mortar. Exact qua	tit f					
		(travel scale on the cartridge) see instruction sheet. Fill the drill hole with mortar, always begin from the bottom of the hole to avoid bubbles						
		Push-through installation: By using Highbond-Anchor rods FHB II-A L the drill hole also filled with mortar. FHB II–A S is this not necessary.	in the fixture must be					
		For drill hole depth ≥ 170 mm use an extension tube						
nsta	allation Highbond-And	hor rod FHB II – A L and FHB II – A S						
)		Only use clean and oil-free anchor rods. Push the anchor rod down to the bottom of the hole, turning it slightly while doing so.						
		After inserting the anchor rod, excess mortar must be en anchor rod	nerged around the					
0		For overhead installations support the anchor rod with wedges. (e.g. fischer centering wedges)						
1		Wait for the specified curing time t _{cure} see Table B6	Mounting the fixture T _{inst} see Tables B2 and B3					
fier	cher Highbond-Anchor	· FHB II	1					
100								



	cteristic values un bond-Anchor FHI			r quas	i-statio	c tens i	on lo	ad for	fische	ər	
			M8x	M10x	M	2x		M16x		M20x	M24x
Size FHB II – A L			60	95	100	120	125	145	160	210	210
Bearing capacity u	nder tensile load, st	eel fai	lure						-		
3	Steel, zinc plated	1	25,1	34,4	49	9,8		96,6		13	7,6
Characteristic	Stainless steel A4	1	,					,.			
resistance N _{Rk,s}	High corrosion resistant steel C		25,1	34,4	49	9,8		96,6	6 137,6		
Partial safety factor	S ¹⁾										
	Steel, zinc plated	1					1,5 ¹⁾				
Partial safety factor	Stainless steel A4						1,5 ¹⁾				
ŶMs,N	High corrosior resistant steel C						1,5 ¹⁾				
Pullout failure in cra	acked concrete C20/2	5									
Characteristic resista	nce N _{Rk,p}	[kN]					3)				
Pullout and splitting	g failure in uncracked	concr	ete C20)/25							
Characteristic resista	nce N _{Rk,p}	[kN]					3)				
Edge distance	C _{cr,sp}	·	300	476	380	600	375	500	580	6	30
Spacing	S _{cr,sp}	[mm]	150	238	190	300	188	250	290	3	15
Pullout and splitting	g failure in uncracked	concr	ete C20)/25							
Characteristic resista	nce N _{Rk,p} ²⁾	[kN]	20	35	40	50	3)	75	95		-3)
Edge distance	C _{cr,sp}	[1,5h _{ef}				
Spacing	S _{cr,sp}	[mm]					3,0h _{ef}				
Factors for the com	pressive strength of	concre	ete > C2	0/25							
	C25/30						1,10				
	C30/37						1,22				
Increasing factor	C35/45						1,34				
for N _{Rk,p}	С40/50 Ψ _с	[-]	1,41								
	C45/55		1,48								
	C50/60		1,55								
Factors acc. to CEN	/TS 1992-4:2009 Sect	ion 6.2	2.2.3								
Uncracked concrete	k _{ucr}						10,1				
Cracked concrete	k _{cr}	[-]					7,2				
Concrete cone failu	re										
Effective anchorage	depth h _{ef}	[mm]	60	95	100	120	125	145	160	2	10
Partial safety factor ¹⁾	5) Үмс	[-]	1,5 ⁴⁾				1	,5			
²⁾ Proof of splitting		1, Ann				stead o	f N ⁰ _{Rk,c}	use N _R	κ,p•		
fischer Highbon Performances	d-Anchor FHB II								Δn	nex (
	es under static or qua	si-stati	ic tensio	on load	for						

fischer Highbond-Anchor FHB II – A L (uncracked or cracked concrete)



sile load, ste I, zinc plated less steel A4 gh corrosion stant steel C		2	75 5,1 5,1	75 34,4 34,4	95 61,6	170	170 8,5	
l, zinc plated less steel A4 gh corrosion		2			61,6	12	8,5	
less steel A4 gh corrosion					01,0	12	0,5	
gh corrosion	[kN]	2	5,1	34.4				
		-	•,•		61,6	12	8,5	
				01,1			0,0	
l, zinc plated				1,	5 ¹⁾			
less steel A4				1,	5 ¹⁾			
igh corrosion stant steel C				1,	5 ¹⁾			
crete C20/2	5							
	1 1				-3)			
		ete C20/25	5					
N _{Rk,p}	[kN]				-3)			
C _{cr,sp}	r		300		340	5	10	
S _{cr,sp}	Imm		150		170	2	255	
n uncracked	concr	ete C20/25	5		,			
N _{Rk,p} ²⁾	[kN]	20	:	25	40		-3)	
C _{cr,sp}	[]			1,5	5h _{ef}			
S _{cr,sp}	լտոյ			3,0)h _{ef}			
strength of a	concre	te > C20/2	25					
25/30				1,	10			
80/37				1,	22			
5/45				1,	34			
0/50 Ψ_{c}	[-]			1,	41			
5/55		1,48						
60/60				1,	55			
4:2009 Sect	ion 6.2	2.2.3						
k_{ucr}				1	0,1			
k _{cr}	[-]			7	7,2			
h _{ef}	[mm]	60		75	95	1	70	
γмс	[-]	1,5 ⁴⁾			1,5			
	igh corrosion stant steel C rete C20/25 $R_{Rk,p}$ n uncracked $N_{Rk,p}$ $c_{cr,sp}$ $s_{cr,sp}$ n uncracked $N_{Rk,p}^{2)}$ $c_{cr,sp}$ $s_{cr,sp}$ strength of c 5/30 5/45 60/50 4:2009 Sect k_{ucr} k_{cr} h_{ef} γ_{Mc} al regulation	$ \begin{array}{c c c c c c c } \hline [-] \\ \hline [c] \hline \hline [c] \\ \hline [c] \hline \hline [c] \\ \hline [c] \hline \hline [c] \\ \hline \hline [c] \hline \hline \hline [c] \hline \hline \hline \hline [c] \hline \hline \hline [c] \hline \hline$	igh corrosion istant steel C[-]igh corrosion istant steel C[-]in uncracked concrete C20/25NRk,p[kN]in uncracked concrete C20/25NRk,p[kN]c _{cr,sp} s _{cr,sp} [mm]in uncracked concrete C20/25NRk,p ² [kN]in uncracked concrete C20/25NRk,p ² [kN]in uncracked concrete C20/25Strength of concrete > C20/2Strength of concrete > C20/2in uncracked concrete > C20/25Strength of concrete > C20/2Strength of concrete > C20/2 <th< td=""><td>igh corrosion istant steel C[-]Igh corrosion istant steel C[KN]In uncracked concrete C20/25NRk,p[KN]Cor,sp Sor,sp[mm]300Sor,spNRk,p²[kN]20150NRk,p²[kN]20150NRk,p²[kN]20150NRk,p²[kN]20150Sor,sp[mm]strength of concrete > C20/2525/30[-]35/45Ψ_c25/55[-]50/60[-]4:2009 Section 6.2.2.3kucr kcr[-]hef[mm]60$\gamma_{Mc}$$\gamma_{Mc}$[-]1,5⁴)al regulations</td><td>igh corrosion stant steel C I⁻¹ 1, increte C20/25 N_{Rk,p} [kN] increcked concrete C20/25 N_{Rk,p} [kN] increcked concrete C20/25 0 0 increcked concrete C20/25 0 0 increcked concrete C20/25 0 0 increcked concrete C20/25 0 25 increcked concrete C20/25 0 25 increcked concrete C20/25 0 increcked concrete > C20/25 increcked concrete > C20/25 </td><td>igh corrosion stant steel C [-] 1,5¹ increte C20/25 1,5¹ NRk,p [kN] 3) increte C20/25 1 1 increte C20/25 1 1 increte C20/25 1 1 increte C20/25 1 1 1 increte C20/25 1 1 1 1 increte C20/25 1 1 1 1 1 increte C20/25 1</td><td>$\begin{array}{c c c c c c c c c c c c c c c c c c c$</td></th<>	igh corrosion istant steel C[-]Igh corrosion istant steel C[KN]In uncracked concrete C20/25NRk,p[KN]Cor,sp Sor,sp[mm]300Sor,spNRk,p ² [kN]20150NRk,p ² [kN]20150NRk,p ² [kN]20150NRk,p ² [kN]20150Sor,sp[mm]strength of concrete > C20/2525/30[-]35/45 Ψ_c 25/55[-]50/60[-]4:2009 Section 6.2.2.3kucr kcr[-]hef[mm]60 γ_{Mc} γ_{Mc} [-]1,5 ⁴)al regulations	igh corrosion stant steel C I ⁻¹ 1, increte C20/25 N _{Rk,p} [kN] increcked concrete C20/25 N _{Rk,p} [kN] increcked concrete C20/25 0 0 increcked concrete C20/25 0 0 increcked concrete C20/25 0 0 increcked concrete C20/25 0 25 increcked concrete C20/25 0 25 increcked concrete C20/25 0 increcked concrete > C20/25 increcked concrete > C20/25	igh corrosion stant steel C [-] 1,5 ¹ increte C20/25 1,5 ¹ NRk,p [kN] 3) increte C20/25 1 1 increte C20/25 1 1 increte C20/25 1 1 increte C20/25 1 1 1 increte C20/25 1 1 1 1 increte C20/25 1 1 1 1 1 increte C20/25 1	$\begin{array}{c c c c c c c c c c c c c c c c c c c $	



	haracteristic valu scher Highbond									concr	ete)	
Size FHB II – A L			M8x	M10x	M12x		M16x		M20x	M24x		
					95	100	120	125	145	160	210	210
Bearing capac	city under tensile lo	oad, ste	el fail	ure								
without lever a	arm		_								_	
	Steel, zinc plated			13,7	20,8	30,3		56,3		87,9	126,9	
Characteristic resistance	Stainless steel A4		[kN]	15,2	23,2	33,7		62,7		97,9	141	
with lever arm	1											
	Steel, zinc plated			31	62	1()5		266		519	896
Characteristic bending moment	Stainless steel A4 and High corrosion resistant steel C	M ⁰ _{Rk,s}	[Nm]	31	62	1(105		266		519	896
Partial safety	factors ¹⁾											
Partial safety fa	γMs,∨	[-]	1,25									
Ductility factor acc. to CEN/TS 1992-4-5:2009 Section 6.3.2.1		k ₂	[-]					1,0				
Concrete pry-	out failure											
Factor k acc. TR029 Section 5.2.3.3 or. k₃ acc.CEN/TS 1992-4-5:2009 Section 6.3.3		k ₍₃₎	[-]		2,0							
Partial safety fa	actors ¹⁾	γ́Мср	[-]					1,5				
Concrete edge	e failure											
Effective length	Effective length of anchor			60	95	100	112	125	14	4	20	00
Calculation dia	meter	d	[mm]	10	12	14		18			25	
Partial safety fa	γмс	[-]	1,5									
1) In absonce	of other national rec	ulation										

¹⁾ In absence of other national regulations

fischer Highbond-Anchor FHB II

Leistungen

Charakteristische Werte für statische oder quasi-statische Querzugbelastung von fischer Highbond- Ankern FHB II – A L (ungerissener oder gerissener Beton)

Annex C 3



	haracteristic valu scher Highbond							concrete)	
			M1	0x	M12x	M16x	M20x	M24x		
Size FHB II – A S				60	75	75	95	170	170	
Bearing capac	ity under tensile lo	oad, ste	el fail	ure						
Without lever	arm							_		
			19,7		27,3	50,8	80,3	114,2		
Characteristic resistance	Stainless steel A4	$V_{Rk,s}$	[kN]	24,1		33,7	62,7	97,9	124,5	
	High corrosion resistant steel C			24,1		33,7	62,7	97,9	141	
With lever arm										
	Steel, zinc plated			62		105	266	519	896	
Characteristic bending moment	Stainless steel A4 and High corrosion resistant steel C	M ^o _{Rk,s} [Nm]		62		105	266	519	896	
Partial safety	factors ¹⁾									
Partial safety factor		γ̂Ms,∨	[-]	1,25						
Ductility factor acc. to CEN/TS 1992-4-5:2009 Section 6.3.2.1		k ₂	[-]	1,0						
Concrete pryo	ut failure									
Factor k acc. TR029 Section 5.2.3.3 or. k ₃ acc.CEN/TS 1992-4-5:2009 Section 6.3.3		k ₍₃₎	[-]	2,0						
Partial safety fa	γ́Мср	[-]	1,5							
Concrete edge	e failure									
Effective length of anchor		l _f	[mm]	60	7	75		170		
Calculation diameter		d	[mm]	10		12 16		25		
Partial safety fa	γмс	[-]	1,5							

¹⁾ In absence of other national regulations

fischer Highbond-Anchor FHB II

Performances

Characteristic values under static or quasi-static shear load for

fischer Highbond-Anchor FHB II – A S (uncracked and cracked concrete)

Annex C 4



Table C5: D	isplace	ment fo	r fischer	Highbo	nd-Anch	or FHB	II – A L					
Size FHB II – A L		M8x M10x		M	12x		M16x	M20x	M24x			
		60	95	100	120	125	145	160	210	210		
Displacement	t under te	ension loa	ad		-	-			-			
Cracked cond	rete											
Tension load	[kN]	6,6	15,9	17,1	22,5	24,0	30,0	34,7	52,2	52,2		
δ _{N0}	[mm]		0	,8				0,6				
δ _{N∞}	- [mm]	1,7										
Uncracked co	oncrete											
Tension load	[kN]	9,3	22,3	24,0	31,6	33,6	42,0	48,7	73,2	73,2		
δ _{N0}	[]	0,2		•	0	,4		0,6				
δ _{N∞}	- [mm]		1,7						•			
Displacement	under s	hear load										
Uncracked or	cracked	concrete										
Steel zinc pla	ted											
Shear load	[kN]	7,8	11,9	17,3		32,2			50,2	72,5		
δ _{V0}	[1	,2	1,3					3,5			
δ _{V∞}	- [mm]	1	,8	2,0					5,3			
Stainless stee	el A4											
Shear load	[kN]	8,7	13,3	19,3		35,8		55,9	80,6			
δ _{V0}	[]	1,0 1,1 2,2								,5		
δ _{V∞}	- [mm]	1	,5	1,7		3,3			5,3			
High corrosio	n resista	nt steel C	;									
Shear load	[kN]	8,7	13,3	19,3		35,8			55,9	80,6		
δ _{V0}	[]	1	,2	1	,3		2,4		3,7	5,0		
δ _{V∞}	- [mm]	1	1,8		,0		3,6	5,6	7,5			

fischer Highbond-Anchor FHB II

Performances Displacement for fischer Highbond-Anchor FHB II – A L



Table C6: D	isplace	ment for fisc	her Highbo	nd-Anchor	FHB II – A S					
Size FHB II – A S		M1	0x	M12x	M16x	M20x	M24x			
		60	75	75	95	170	170			
Displacement	under te	ension load								
Cracked conc	rete									
Tension load	[kN]	6,6	1 [.]	1,1	15,9	38,0				
δ _{N0}	— [mm]	0,8	0	,3	0,4	0,6				
δ _{N∞}	— []	1,7								
Uncracked co	ncrete									
Tension load	[kN]	9,3	1:	5,6	22,3	53,3				
δ _{N0}	— [mm]		0,2 0,5							
δ _{N∞}	— []				1,7					
Displacement	under sl	hear load								
Cracked or ur	ncracked	concrete								
Steel zinc plat	ted									
Shear load	[kN]	11,3		12,7	12,7 29,0		65,3			
δ_{V0}	— [mm]	1,2		1	,5	2,8				
$\delta_{V^{\infty}}$	[[]]]]	1,	8	2	2,3	4,2				
Stainless stee	el A4									
Shear load	[kN]	13	,8	19,3	35,8	55,9	71,1			
δ_{V0}	— [mm]	1,	0	1,1	2,2	3	,5			
δ _{V∞}	— [mm]	1,	5	1,7 3,3		5,3				
High corrosio	n resista	nt steel C								
Shear load	[kN]	13,8		19,3	35,8	55,9	80,6			
δ _{V0}	[mm]	1,	2	1,3	2,4	3,7	5,0			
δ _{V∞}	— [mm]	1,	8	2,0	3,6	5,6	7,5			

Performances Displacement for fischer Highbond-Anchor FHB II – A S

Annex C 6